Research on Reasonable Thickness Design of Expressway Pavement

Structure Based on Grey Relation Analysis of Subgrade Soil

Improvement

1

# **Commented [T1]:** AU: Here is a listing of the types of issues corrected in this paper:

- 1.Passive tense changed to active.
- 2.Usage of articles
- 3.Correcting non-idiomatic word choices
- 4.Reorganizing sentences for simpler, more direct expression
- 5.Removing unnecessary capitalizations
- 6.Consistent hyphenation of compound terms
- 7.Sentence punctuation
- 8.Plurals and subject-verb agreement
- 9.Standard style for units of measure and variables

## Abstract:

During the design of pavement structures, determining the reasonable thickness for pavement layers is normally rather important and also difficult. As to the designing an expressway in areas with poor soil foundation, a reasonable subgrade treatment will help to build a-more durable pavement. However, determining the thickness of subgrade treatment is always a hard work difficult task for the designer. Thicker treatment means to a huge cost increase of for the project, while whereas thinner treatment can-not show a achieve significant improvement of in the mechanical behavior of pavement structures. this manuscript, This study used the finite -element method was utilized to analyze the mechanical response of a real field pavement, which with had experienced a subgrade treatment at a certain depths. The orthogonal The study used orthogonal design and grey relational theory were used to analyze the design indicators and make a better design on for the pavement structure of a field expressway. The numerical calculation index and theoretical analysis results can fully show that the treatment depth of subgrade soil has significant influence on the stresses within an of asphalt pavement structure and the bottom tensile strains of the asphalt layers. Therefore, in order to design a pavement structure with equal structural strength, using a reasonable depth for the cement-treated depth of subgrade, instead of increasing the asphalt layer's thickness, will be a more cost-effective solution.

Key-words: Subgrade subgrade Soil Improvementimprovement; Grey-grey Relational relational Analysisanalysis; Orthogonal orthogonal Testtest; ANSYS; Reasonable reasonable Thickness;

2

**Commented [T2]:** AU: In most cases, academic publications prefer active tense over passive tense.

## 1 .Introduction

At present, in the construction process of highway subgrade, builders often encounter the phenomenon of stress concentration caused by insufficient subgrade soil layer is often encountered, which causes early damage of to the pavement structure.-\_In engineering, a common technical measures is using lime to treat bad subgrade sections to improve soil-based elastic modulus., The effect of the treatment has been recognized by the road industry designers in the industry [1-3]. However, In terms of Pavement Stress and strain design index, factors such as lime treatment, roadbed depth, and reasonable thickness of each layer of pavement structure have great influence on the pavement stress and strain design index it [4-6]. So far, There is little research has been conducted on the relationship between the depth of limetreated soft soil subgrade and the internal stress of asphalt pavement structures.<sup>5</sup> The lime-treated soft soil subgrade in the actual project is blind and random. For example, in the design and construction, regardless of the thickness of the pavement structure layer of the asphalt, a treatment depth is selected according to the experience to improve the subgrade soil. As a result, the rebound modulus of the top of the road is too small, and the pavement structure is easily damaged. When the processing depth is too large, it causes economic waste. Therefore, this study investigates according to the characteristics of the pavement structure, the relationship between subgrade depth treated with lime and pavement structure according to the characteristics of the pavement structure is studied. It is particularly important to determine the reasonable depth of lime\_-treated soil to guide design and construction [7-9]. Therefore, this paper takes the Changchun-Fuyu Expressway reconstruction and expansion project as an example. Through indoor and outdoor experiments and finite--element (FE) numerical simulation analysis, we studied the influence of the depth of lime treatment for the internal stress and strain of the pavement structure-is-deeply-studied, and proposed the reasonable thickness of the pavement structure of each layer is proposed to the for analysis of using grey correlation theory [10-12]. This is The results of this study hold great practical significance of for improving design specifications, ensuring road engineering quality, and saving engineering construction costs.

**Commented [T3]:** AU: In this sentence, do you mean that the choice or selection or the depth of the lime-treated soft soil subgrade is blind and random? Please clarify.

**Commented [T4]:** AU: Whose experience is being used to make this selection?

2 FE model of pavement structure

The <u>study used the FE</u> software ANSYS17.0 was used for numerical simulation calculation. The FE analysis was carried out for a given flexible base asphalt pavement structure. The orthogonal test design method was used to analyze the factors affecting the mechanical response of the pavement structure.

## 2.1 Loads and forms of action

The pavement model uses a 100\_KN single-axis double wheel as the design axle load, and the calculated axle load parameters are determined according to Table 1.

	Table 1: Design	i axle load parameters	
Design of axle load (KN)	Tire ground pressure (Mpa) +	Equivalent radius of single wheel grounding (mm) +	Two wheel center distance (mm) $\varphi$
100+2	0.70	213.00	319.5¢

## 2.2 Pavement geometry model and boundary conditions

Fig.\_1 shows the pavement structure used for FE calculation. The geometry of the 3D model is  $10 \times 7.5 \times 4$  m. In the typical structure of the traditional pavement, a cold recycled asphalt ATB-25 flexible base layer is added to form a composite pavement structure [13-15]. The material parameters and thickness parameters are shown in Table 2.



Figure 1: Asphalt pavement structure model and grid division diagram

4

**Commented [T5]:** AU: The units for tire ground pressure should be capitalized thusly: MPa This should be corrected in later tables as well.

**Commented [T6]:** AU: The images you created of your tables show the nonprinting characters (such as the arrows for soft returns at the end of lines). This should be corrected prior to submission for publication.

Project₽	roject $\circ$ Material type $\circ$ Modulus Poisson's ratio $\circ$ Scheme					thogonal exp	erimenta	l data model	ing⇔	ę
Asphalt	Stone Matrix Asphalt with Modified Bitumen (SMA-13)~	10000 Mpa-	10000 Mpa <sup>o</sup> 0.25°		100	40mm* <sup>3</sup>	110	50mm₀	120	ę
surface₽	Mesograin asphalt mixture modified asphalt concrete (AC-20). <sup>o</sup>	11500 Mpa	0.25*	60mm¢	mm↔	70mm*'	mmø	70mm₄	mm₽	ø
ATB-25 base layer@	Dense-graded cold recycling bituminous mixture (ATB-25)	9000 Mpa-	0.25*	80mm.₀	80 mm.	120mm*	120 mm.	130mm∘	130 mm.	ę
Inorgan ic Binding	Cement treated base 7‰	11500 Mpa	0.250	300mm.	500	380mm.	580	380mm₽	680	ę
Materia lse	Cement treated base 5%	8000Mp a+3	0.25¢	200mm@	mm.∘ m.∘ 200mr		mm₽	300mm₽	mm₽	ø
	Subgrade	40Mpa₊	0.4~		Subgrade 40Mpa 0.4 Improved subgrade modulus 120Mpa					ہ ہ

Table 2: Pavement structure layer thickness and material composition.

For convenience of analysis, the pavement FE model is based on four boundary conditions: (1) The-the surface of the pavement structure is used as a free surface without any constraints; (2) The model has no displacement in the  $X_x$ -axis direction of the two sides along the advancing direction of the vehicle; (3) There-there is no displacement in the  $Y_y$ -axis direction on both sides in the width direction; and (4) The-the bottom surface of the model has no displacement along the  $Z_z$ -axis. For the asphalt pavement structure, considering the time that the vehicle load acts on the asphalt pavement is very short, it is feasible to analyze the asphalt layer and the cement-treated layer as a linear elastomer.

## **3** Determination of control indicators and program

## 3.1 Determination of control indicators

According to the latest asphalt pavement design specifications in China, anti-deformation performance and anti-fatigue cracking performance of asphalt concrete are two important design indicators for asphalt pavement design-in-China. Therefore, the difference in depth of subgrade treatment will affect the performance of the road surfaces against rutting, and the shear stress is the main factor causing the rutting diseaseproblem. Therefore, So we take the shear stress of the asphalt layers and the asphalt stabilized base layer are taken as an-indicators. In addition, the bottom

5

Formatted: Font: Italic, Complex Script Font: Italic

Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic

Formatted: Font: Bold, Complex Script Font: Bold

tensile strain of the asphalt layer and asphalt stabilized base layer, and as well as the bottom tensile stress of the cement\_-treated base layer are also important indicators reflecting the anti-fatigue cracking performance of the structural layer. For this reason, we also include these three indicators are also include in the stress control index system.

## 3.2 Determination of Orthogonal orthogonal factors determination

There are many factors affecting the structural performance of asphalt pavement. We selected four factors as the influencing factors of the stress calculation: The (1) the improvement depth of subgrade soil, (2) the thickness of the asphalt layer, (3) the thickness of the asphalt\_-stabilized base layer, and (4) the thickness of Cementcementtreated base layer are selected as the influencing factors of stress calculation. We measured Each each factor was taken at three levels; and from this data obtained a four-factor, three-level test form was obtained. In order to efficiently find out the influence of soil depth on the structural stress of pavement, this paper intends to uses the L9 (3<sup>4</sup>) orthogonal test analysis method to analyze and calculate the influencing factors. Factor levels and orthogonal experimental schemes are shown in Tables 3 and 4.

÷			Table 3: F	actors and levels.		
				Factor		¢₽
	Level.	A Improvement depth of subgrade soil (cm)+2	B Asphalt layer thickness (cm).	C+/ ATB-25 base thickness(cm)+/	D Cement treated base & thickness (cm)&	Ģ
	10	2000	1040	8043	500+	ę
	$2^{\varphi}$	5000	110	120+7	580÷	ę
	3.0	8000	12.0	130-	680+2	ę

6

Formatted: Font: Bold, Complex Script Font: Bold Formatted: Font: Bold, Complex Script Font: Bold

**Commented [T7]:** Is this selection made for this study specifically or more generally within the industry? Note the passive tense here. My edit corrects passive tense, but to do so I had to add the agent who made this selection. Am I correct that this was a choice made by the researcher?

Formatted: Font: 12 pt, Font color: Black

		Tabl	e 4: Orthogonal	scheme₽			
Model₊	Stone Matrix Asphalt with Modified Bitumen (SMA-13) (m) +	Mesograin asphalt mixture modified asphalt concrete (AC-20) (m)	Dense-graded cold recycling bituminous mixture (ATB-25)(m).	Cement treated base 7% (m) e	Cement treated base 5% (m) e	Improvement depth of subgrade soil (m) e	ب ب
10	0.04	0.06+	0.08	0.3+2	0.2*	0.2+	- 
20	0.040	0.070	0.12+2	0.38	0.2+2	0.2+2	- +
3⇔	0.05~	0.070	0.130	0.38	0.3+2	0.2+2	- +
4₽	0.04~	0.06	0.12+	0.38	0.2+2	0.5+	- 
5₽	0.040	0.070	0.13+2	0.38	0.3+2	0.5+	- 
6⇔	0.050	0.070	0.08+2	0.3+2	0.2~	0.50	- +
7⊷	0.040	0.06+2	0.13+2	0.38	0.3+2	0.84	- 
8.0	0.040	0.070	0.08+2	0.34	0.2+2	0.8+2	- +
9₽	0.050	0.070	0.12	0.38+2	0.2+2	0.8+	- 

#### 4 Grey relational calculation and analysis

<u>This study uses As a reference factor of the design of highway asphalt pavement</u> structure, the asphalt layers shear stress, asphalt stabilized base shear stress, asphalt layer tensile strain, asphalt stabilized base tensile strain, and inorganic binder stabilized base layer tensile stress <u>as reference factors for the design of highway</u> <u>asphalt pavement structures. are used</u>. The orthogonal experimental model is used to solve the gray correlation degree <u>between among</u> soil depth<u>a</u> and-shear stress<u>a</u> and tensile stress\_[17-21]. The main control indicators for the depth of ground improvement are determined mathematically.

This paper takes the typical design structure <u>of for an</u> expressway in Northeast China as the basis of analysis<sub>25</sub> The shear stress of the asphalt surface layer, the layer bottom strain of the asphalt mixture, the tensile strain of the flexible base layer, and the tensile stress of the inorganic binder layer are taken as design parameters. Mathematical grey relational analysis method is used to determine the main control indicators of subgrade processing depth.

#### 4.1 Interval processing of raw data

According to the orthogonal scheme, nine test results are calculated for each control index, such as defining shear stress of asphalt upper layer, as follows:

 $X_{1} = \{x_{1}(1), x_{1}(2), x_{1}(3), x_{1}(4), x_{1}(5), x_{1}(6), x_{1}(7), x_{1}(8), x_{1}(9)\} = \{125600, 115190, 133460, 113510, 122980, 115060, 121260, 116160, 127240\}$ 

All the <u>rests-others</u> are determined in the same way, <u>and</u> the calculation results are summarized in Table 5.

7

**Commented [T8]:** AU: Since there are three items listed here, I have replaced "between" with "among." If the items are compared in a pairwise fashion, then you may use "between," but it would help to clarify the method of comparison.

Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Complex Script Font: Italic
Formatted: Indent: First line: 0 ch

T	able	5: 5	Stress o	calculat	ion res c	ults and olumn :	l gray co summar	orrelatio y table₊	on calcu	ilation o	definitio	n data	
							k.₀						
Parameter name.	$\mathbf{X}_{i}$	(k)₽	10	2.0	3₽	40	5₽	6⇔	7₽	8+3	9⇔	Xmin(i)~	Xmax(i)~
Subgrade													
improvement		0,0	0.24	0.50	0.80	0.20	0.5+2	0.80	0.2+2	0.50	0.80	0.2+2	0.8
depth (m) e	_												
SMA-13 shear			1256	1151	1334	1135	12298	1150	1212	1161	12724	110510	100460
stress (pa) @		10	0040	90₊∂	60₽	10+2	0,0	60+2	60⇔	60+2	0,0	1135100	133460
AC-20 shear	-	_	2486	2694	2773	2617	27207	2600	2407	2590	27146		
stress (pa) 🧧		20	100	104	20¢	40.0	0,0	70₽	800	304	040	240780	277320+
ATB-25 base	-		2096	2127	1956	2212	21128	1854	2018	2005	19248		
layer shear stress (pa) २	į₽	3₽	000	300	70₽	80,0	043	200	60₽	80,0	0,0	185420	221280
Asphalt layer	-		1.47	1.44	9.46	1.54	1.21E	9.20	1.52	1.18	9.55E		
bottom tensile strain₀		4₽	E-08+	E-08₽	E-09+	E-08.0	-080	E-09¢	E-08¢	E-08¢	<b>-</b> 09¢	9.2E <b>-</b> 09∻	1.54E-08∉
ATB-25 layer	-		5.00	3.62	1.66	2.08	1.70E	2.35	1.79	2.44	1.78E	1.66E-0	
bottom strain.		5₽	E-06₽	E-06+2	E-06+2	E-06₽	-06	E-06+2	E-06+	E-06+2	-06+2	6₽	0.000005+
Cement treated	-												
base layer			1341	7357	6991	7749		1068	5470	1122			
bottom tensile		6₽	30¢ <sup>2</sup>	7₽	5₽	10	72252₽	30+2	3₽	10+2	67896₽	54703~	134130~
stress (pa)													

**Commented [T9]:** AU: In Table 5, the abbreviation for Pascals should be capitalized (Pa). Also, variables such as  $X_i(k)$  should be in italics.

The number of values after the interval value processing is calculated according to  $\frac{\text{formula}\text{Formula}}{(1)}$ , and the results are summarized in Table 6.

$$y_i(\mathbf{k}) = \frac{\mathbf{x}(\mathbf{k}) - \mathbf{x}_{\min}(\mathbf{k})}{\mathbf{x}_{\max}(\mathbf{k}) - \mathbf{x}_{\min}(\mathbf{k})}$$
(1)

8

	Ta	able 6: S	Summary	of the s	eries afte	er the int	erval val	lue proce	essing⊬	
	a					k₽				+
Y i(	<u>(</u> K)₽	10	240	3.₽	40	5₽	6.0	7₽	8,	9 <sub>€</sub> , *
	0,0	0.0000¢	0.50000	1.0000+2	0.0000€	0.50000	1.000043	0.0000+2	0.50000	1.0000+2 *
	10	0.6060¢	0.0842	1.0000+2	0.000€	0.4747	0.0777~	0.3885¢	0.1328	0.6882**
	2÷	0.2143¢	0.78350	1.0000+2	0.5736	0.8563.	0.5279.0	0.0000+2	0.4995¢	0.8396**
į₽	3₽	0.6743@	0.7616	0.2858	1.0000+2	0.7211.0	0.0000،	0.4584	0.4228	0.1969*
	4₽	0.88710	0.8387.0	0.0419	1.0000+2	0.4677.	0.000040	0.9677₽	0.4194.	0.0565**
	5₽	1.0000¢	0.5868.	0.0000+2	0.1257.	0.0120+	0.2066.	0.0389	0.23350	0.0359**
	6₽	1.0000+2	0.2376	0.1915	0.2869	0.2209	0.6563	0.0000₊ <sup>3</sup>	0.7240	0.1661*

## 4.2 Difference sequence

The absolute difference between each index of the object to be evaluated (comparison sequence) and the corresponding element of the reference sequence is calculated one by one.

$$|\mathbf{x}_{0}(\mathbf{k}) - \mathbf{x}_{i}(\mathbf{k})|$$
 (k=1,...,9, i=1,...,6).

Table 7: Summary of difference sequence.

	(1-)					k.₀					$\Delta_{\min}$	∆ <sub>max</sub> +
∆ <mark>1</mark> (к)⊬		10	240	3₽	4₽	5₽	5÷ 6÷		80	9₽	(k) 🖉	(k) 🐖
	10	0.6060₽	0.41580	0.0000¢ <sup>3</sup>	0.000043	0.0253	0.9223	0.38850	0.3672+	0.3118	0.0000	0.92230
	20	0.2143	0.2835@	0.0000¢ <sup>3</sup>	0.5736	0.3563	0.4721	0.000040	0.0005	0.1604	0.0000	0.5736
	3₽	0.6743	0.26160	0.7142+2	1.0000	0.2211.0	1.000040	0.45840	0.0772+2	0.8031	0.0772	1.0000
Ĵ₀.	4₽	0.8871	0.3387@	0.9581	1.0000	0.0323	1.000040	0.9677~	0.0806	0.9435	0.03230	1.0000
	5⊷	1.0000	0.0868@	1.0000+3	0.1257	0.4880	0.7934	0.0389	0.2665+	0.9641@	0.03890	1.00000
	6₽	1.0000	0.2624@	0.8085	0.2869	0.2791	0.3437	0.0000	0.2240	0.8339+	0.0000*	1.0000.

4.3 Finding the maximum difference and the minimum difference between the two<sup>\*</sup> poles of each column.

The range is calculated according to the <u>formulaFormula</u>—(-(3))—and the <u>formulaFormula</u>—((4))—. As can be seen from Table 8, min  $\Delta i(k) = 0$ , max  $\Delta i(k) = 1$ .

$$\min \Delta_{i}(k) = \min_{i=1}^{6} \min_{k=1}^{9} |x_{0}(k) - x_{i}(k)|$$
(3)  
$$\max \Delta_{i}(k) = \max_{i=1}^{6} \max_{k=1}^{9} |x_{0}(k) - x_{i}(k)|$$
(4)

4.4 Calculate Calculating the correlation coefficient

According to the formula <u>-((5)</u>-

$$\zeta_{i}(k) = \frac{\min_{i} \min_{k} |x_{0}(k) - x_{i}(k)| + \rho \cdot \max_{i} \max_{k} |x_{0}(k) - x_{i}(k)|}{|x_{0}(k) - x_{i}(k)| + \rho \cdot \max_{i} \max_{k} \max_{k} |x_{0}(k) - x_{i}(k)|}$$

**Commented [T10]:** AU: In Table 6, variables should be in italics.

Formatted: Font: Italic, Complex Script Font: Italic Formatted: Complex Script Font: Italic Formatted: Indent: First line: 0 ch

(2)

Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Commented [T11]: AU: In Table 7, variables should be in
italics, except for the subscripts min and max and the
Gamma character.

Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Indent: First line: 0 ch
Formatted: Complex Script Font: Italic
Formatted: Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Indent: First line: 0 ch
Formatted: Complex Script Font: Italic

$$k = 1, \cdots, 9$$
 (5)

The correlation coefficients of the corresponding elements of the comparison sequence and the reference sequence is are respectively calculated, respectively, where  $\rho$  is the resolution coefficient and take the value in (0,1). If the  $\rho$  is smaller, the difference between the correlation coefficients is larger, and the discrimination ability is stronger. In this paper,  $\rho$  is taken as 0.5, and the calculation results are summarized in Table 8.

Table 8: Summary of correlation coefficient and relevance.

			<b>Table 8</b> : Summary of correlation coefficient and relevance.										Com	mented [T12]: AU: In Table 7, variables should be in
Ĕ	( <b>b</b> )					k₽					Correlation	Proport	italics	
<u> </u>	( <b>K</b> )+	1.0	2₽	3∻	4₽	5∻	6₊₂	7₽	8,	9₊₂	Xm~	of facto	orse e	
	10	0.4521@	0.5460	1.0000	1.000043	0.9518	0.3515+	0.5628	0.5766	0.6159	0.6730	0.187	ہ ہو	
	2,0	0.7000	0.6381.	1.0000	0.4657	0.5839	0.5144	1.000043	0.9989¢	0.7571+2	0.7398	0.206	6~ ~	
	3.0	0.4258@	0.6565	0.4118	0.33334	0.6933	0.33334	0.5217	0.8662+2	0.3837	0.5140	0.143	5e e	
<b>Ĵ</b> ⇔	40	0.3605	0.5962+2	0.3429	0.333342	0.9394	0.333342	0.3407	0.8611	0.3464	0.4949	0.138	2~ ~	
	5₽	0.33334	0.85200	0.33334	0.7990	0.5061	0.3866	0.9278	0.6523+	0.3415+	0.5702+2	0.159	ء م	
	6,0	0.3333*	0.6558	0.3821+	0.6354	0.6418	0.5926	1.0000	0.6906	0.3748	0.5896	0.164	6e e	

The correlation coefficient between the corresponding elements of the comparison sequence and the reference sequence is ranked according to the correlation coefficient of the soil depth improvement according to various factors:  $0.7398 \ge 0.6730 \ge 0.5896 \ge 0.5702 \ge 0.5140 \ge 0.4949$ , and  $y_2 \ge y_1 \ge y_6 \ge y_5 \ge y_3 \ge y_4$ can be obtained. -

4.5 Calculat	<i>e the proportio</i> Table 9:	n of the importe Summary of the	<i>ance of each fa</i> proportion of e	<i>ctor</i> ach factor₊	•
SMA-13 shear stress (pa).	AC-20 shear stress (pa)∘	ATB-25 shear stress (pa)	Asphalt mixture layer bottom strain.	ATB-25 base layer bottom strain. <sup>2</sup>	Cement treated base layer bottom tensile stress (pa)
18.79‰	20.66%	14.35%	13.82%	15.92%	16.46‰

## 5 Analysis of Orthogonal orthogonal experimental results analysis

In this paper, we need to examine four test factors at the same time. If a comprehensive test is carried out, the scale of the test will be large, and it is often difficult to implement due to the limitations of the test conditions. Orthogonal tested to-design is a high-efficiency test design method that arranges multi-factor tests and seeks optimal combination of levels.

According to the results of gray correlation analysis, the orthogonal test method

Formatted: Font: Italic, Complex Script Font: Italic Formatted: Font: Italic, Complex Script Font: Italic

Formatted: Font: Italic, Complex Script Font: Italic

Commented [T13]: AU: Please confirm the use of

subscripts here.
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Subscript
Formatted: Font: Italic, Complex Script Font: Italic
Formatted: Indent: First line: 0 ch
Formatted: Complex Script Font: Italic
<b>Commented [T14]:</b> AU: Pascals should be abbreviated as Pa.

is used to distinguish the primary and secondary order of the influence of various influencing factors on the mechanical indicators, and to find out the optimization plan, (that is, what level of each factor is considered to meet the requirements of the test indicators)<sub>r<sub>a</sub></sub> <u>Analyze analyze</u> the relationship between factors and indicators, and find out the rules and trends of indicators with factors.

1) Determine Determining the calculation coefficient  $b_{ki}$ 

Calculate the variation range of each indicator, that is the difference <u>D</u> between the maximum value and the minimum value, and calculate the correlation coefficient by  $b_{ki}$ =ratio/<u>D</u>.

The results are summarized in Table 10:

 Table 10: Range of variation of various indicators and calculation of correlation

 coefficient table.

Index₽	SMA-13 shear stress (pa) <sub>*</sub>	AC-20 shear stress (pa).	ATB-25 shear stress (pa).	Asphalt mixture layer bottom strain <sub>t</sub> ,	ATB-25 base layer bottom strain.	Cement treated base layer bottom tensile stress (pa),	*
Range of variation D₀	19950*	36540.	358600	6.2E <b>-</b> 09¢	3.34E-06₽	79427.	4
bkit?	9.41876E-06	5.6531E-06+	4.00192E-06∉	22286104.84 <sub>e</sub>	47669.75991	2.07274E-06	4
	2) Compre	ehensive evalua	tion of paveme	nt performanc	e	•	_

According to  $Y_{k} = \sum b_{ki} \times X_{ki}$  ( $K = 1, \dots, 9; i = 1, \dots, 6$ ) [16], the performance index of each FE model is calculated and the results are filled given in Table 11.

**Commented [T15]:** AU: Unclear transition here. Are you starting a list of steps in the test design? Please add a statement just prior to this to explain for readers what is coming next.

Formatted: Font: Italic, Complex Script Font: Italic Formatted: Font: Italic, Complex Script Font: Italic Formatted: Font: Italic, Complex Script Font: Italic Formatted: Font: Italic, Complex Script Font: Italic

Formattad: Indent: Laft 0 ch				
Formatted: Indent: Lett 0 ch				
Commented [T16]: AU: This number in brackets is meant				
to be a reference citation, correct? Thus, it would not be				
correct to have this as a superscript.				
Formatted: Font: Italic, Complex Script Font: Italic				
Formatted: Font: Italic, Complex Script Font: Italic				
Formatted: Font: Italic, Complex Script Font: Italic				
Formatted: Font: Italic, Complex Script Font: Italic				
Formatted: Font: Italic, Complex Script Font: Italic				
Formatted: Not Superscript/ Subscript				

Table	e <b>11</b> : Orth	10gonal test p	erformance	index sum	mary table	<b>2</b> ₽	
		$A_{i}$	B₽	$\mathbf{C}_{e^2}$	$D_{t^2}$	Test results.	
Test number.		1.	2+2	3₽	40	Performance index	
1.0		10	10	1.0	10	4.27₽	
2,,		10	2.0	20	20	4.11.0	
3.0		10	3.0	3₽	3₽	4.040	
4.0		2.0	10	20	3.0	4.040	_
5+3		20	20	3+2	1.0	4.040	
6 <sub>4</sub> 2		2.0	3.0	10	2.0	3.83+	_
7.∞		3₽	10	3₽	20	3.85+	_
8.0		3₽	2.0	10	3₽	3.97+	
943		3.0	3.0	20	10	3.94*	_
	$K1_{\phi}$	12.4191	12.1570+	12.0786	12.2553+	نه	_
Level sum₽	K2¢	11.9141.	12.1206	12.0843	11.7883.	ب T=36 006	
	K3¢	11.7633	11.8189.	11.9336	12.05290	4 <sup>-50.090</sup> *	
	k1₽	4.1397 <i>v</i>	4.0523	4.0262*	4.0851.0	P=144.77↔	
Mean Value,	k2+	3.97140	4.0402	4.0281	3.9294	Q=144.92+	
	k3+	3.9211¢	3.9396	3.9779₽	4.0176	ب 8⊤=0.1431ب	
Range Rø		0.2186 <i>0</i>	0.1127@	0.0502~	0.1557.	e -	
Factors sum of squares Se		0.0787	0.0230+2	0.0049	0.0366	Ģ	+





<u>Since Because</u> the degrees of freedom of ABCD are the same, both are  $r_{--1} = 2$ , so the squared size can be used to represent the mean square size to judge the relative size of each factor (see Figure 2)[17].

It can be seen from Fig. 4 that the main order of the factors (main  $\rightarrow$  sub) is as <u>follows</u>: ADBC, the depth of the subgrade soil improvement  $\rightarrow$  the thickness of the inorganic binder stabilized base layer  $\rightarrow$  the thickness of the asphalt layer  $\rightarrow$  the thickness of the ATB base layer.

Then draw the relationship between various factors and the road performance index (see Figure <u>5</u> 3 <u>-through</u> Figure <u>6</u>).



Figure 3: Effect of subgrade soil improvement depth on pavement performance index



Figure 4: Influence of asphalt layer thickness on pavement performance index

Formatted: Font: Italic, Complex Script Font: Italic

Formatted: Not Superscript/ Subscript

**Commented [T17]:** AU: This statement is a command. Should this be part of the numbered list?



Figure 5: Effect of ATB-25 base thickness on pavement performance index



Figure 6: Effect of cement\_treated base layer thickness on pavement performance index

The thickness of each structural layer of asphalt pavement is one of the important indexes of pavement design. <u>If The the</u> thickness of <u>the</u> pavement structure layer is too thin, the internal stress and strain of the structure is too large, <u>and or</u> the pavement structure layer is too thick, the pavement cost will be too high and uneconomical.

Therefore, in the case of a certain modulus parameter of each structural layer, finding a reasonable thickness is an important part of the pavement structure design.

According to the uniform and comparable properties of the orthogonal test

**Commented [T18]:** AU: If any one of these factors is not optimal, would it not cause the pavement cost to be too high? Thus, shouldn't you use "or" rather than "and" here?

design, it-two observations can be seen from the above data and diagram:

(1) The thickness of the ATB-25 base layer has little effect on the stress and strain of the asphalt pavement structure, indicating that the increase in the asphalt pavement layers and the thickness of the ATB-25 base layer on the overall performance of the pavement is not as high as the increase in the cost. The design can be selected with reasonable thickness according to the relevant specifications. The subgrade improvement depth has a great influence on the stress and strain of asphalt pavement structure. When the subgrade improvement depth is 200 cm, the performance index is 4.1397; when the subgrade improvement depth is 500\_cm, the performance index is 3.9714; when the subgrade improvement depth is 200 cm, the performance index is 3.9211. It shows that with the increase of the subgrade improvement depth, the strength of the subgrade is improved, and the overall variation of stress and strain inside the pavement structure is gradually reduced. However, as the depth of the subgrade treatment increases, the internal stress and strain reduction of the pavement structure is obviously slowed down. The thickness of the inorganic binder\_-stabilized the base layer also has an effect on the stress and strain of the asphalt pavement structure. As the thickness of the inorganic binder layer increases, the internal stress and strain of the pavement structure decreases first and then increases. When the thickness of the inorganic binder is 580\_-cm, The internal stress and strain values of the pavement structure are the smallest.

-(2) The smaller the stress and tensile strain, the better the structure and material combination. Therefore, the smallest value of K1, K2, and K3 is selected, and the optimal combination is A3 B3 C3 D2.<u>when When</u> the modified subgrade soil depth is 800\_cm, the asphalt layer thickness is 12\_cm, the ATB-25 base layer thickness is 130\_cm, and the inorganic binder<u>stabilizes\_stabilized\_the</u>base layer thickness is 580\_cm, it is the optimal structure and material design combination. Compared with the conventional design method, the stress and layer tensile strain obtained by the gray correlation analysis method are smaller, and the obtained structure and material combination are more reasonable. Considering that the asphalt layer thickness and the thickness of ATB-25 have little effect on the internal stress and strain of the pavement structure, from the perspective of cost performance, the final structural scheme of the Changyu Expressway Reconstruction Project takes 11\_cm <u>of\_for the\_asphalt layer</u> thickness.

**6** Conclusions

**Commented [T19]:** AU: Please check this measurement. It repeats a value given previously in this sentence.

(1) The depth of subgrade soil improvement has the most significant influence on the stress of asphalt pavement structures and the bottom strain of <u>the</u> asphalt layer. Secondly, in order of significance is the thickness of the cement\_treated base layer, and again\_followed by the thickness of the asphalt layer, and the <u>The</u> influence of the thickness of the ATB-25 base layer is the smallest.

(2) The optimum structure and material designs combination of the asphalt pavement from Changchun<u>-to-</u>Fuyu Expressway is selected when the depth of soil improvement is 800\_cm, the thickness of asphalt layer is 11\_cm, the thickness of ATB-25 base layer is 120\_cm, and the thickness of cement<u>-</u>treated base is 580\_cm.

(3) According to the previous design ideas, the stress and strain of asphalt pavement structures gradually decrease with the increase of the depth of the structure layer. According to the results of <u>the</u> orthogonal test, <u>the increase-increasing of</u> the thickness of <u>the</u> asphalt layer has little effect on the stress and strain of the pavement interior, and the reasonable thickness of cement\_-treated material needs to be calculated to be determined-finally.